

Traffic Simulation Tests with Road Reflectors

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Abstract

Passively lighted reflector systems are used increasingly for improving pedestrian security. They are glued on the road surface in front of pedestrian crossings and are exposed to any kind of traffic loading. In order to achieve durable and reliable functioning of these reflectors, the mechanical behavior of the road reflectors themselves, as well as the contact zone between the reflectors and the road surface under alternating wheel loading plays a critical role. This paper deals with the results of a recent study at the Swiss Federal Laboratories for Materials Testing and Research (EMPA), where a newly developed type of reflector system (produced by the company Technical Advice & Consulting AG, TAC, and referred in the following as HMB-reflector) has been tested. The reflector system consisted of the reflector with special glue and a cover material. A one third scale model mobile load simulator (MMLS3) was used to investigate the traffic load performance of the reflector-road construction for both old and new compacted asphalt pavement specimens at two different elevated constant temperatures. It was observed that the contact zone between the reflector and surface of the new specimen was destroyed in the rutting zone after some thousands of cycles. In order to determine the position and shape of cavities and debonding effects in one tested specimen, infrared (IR) thermography measurements were carried out. In addition to the MMLS3 traffic simulation tests, resistance to temperature cycles was tested on an old compacted asphalt pavement specimen with the aim to investigate the adhesive behavior of the reflectors and cover material after 20 temperature cycles. Quasistatic pull-off tests were carried out on all reflectors except for the reflectors on the new asphalt specimen.

Keywords

Pavement markings – Pedestrian safety – Repeated loads

1. Introduction

The objectives of the research study were as follows:

- Investigate and assess the mechanical behavior of the glued HMB-reflectors, the 2-component HMB-glue and the 2-component HMB-cover material at different hot weather temperatures with third scale model mobile load simulator MMLS3 (produced by MLS Test Systems cc) on a new and an old asphalt concrete pavement.
- Investigate the resistance to temperature cycles of the HMB-reflector which was glued onto the road surface.
- Carry out quasistatic pull-off tests on all reflectors and evaluate their results with respect to the durability of the combined reflector-road construction.
- Evaluate the suitability of MMLS3 for reflector testing and the possibilities of non-destructive IR thermography damage detection in this special case.

The following tests were performed to achieve the objectives:

- For traffic load performance testing with MMLS3, two compacted asphalt pavement slabs were used: Series A (old pavement) and Series B (newly compacted pavement). In Series A the HMB-reflectors were tested at two elevated constant temperatures $T=30^{\circ}\text{C}$ (experiment A30) and $T=40^{\circ}\text{C}$ (experiment A40). In Series B tests were conducted at one temperature $T=40^{\circ}\text{C}$ (experiment B40). Both Series A and B were tested under ambient relative humidity conditions. Two HMB-reflectors were applied for every experiment. One reflector was arranged perpendicular to the longitudinal axis of MMLS3 whereas the other one was located in an angle of 45° to this axis. The vertical deflections of the specimen surface were measured and recorded periodically during the tests.
- In order to investigate resistance to temperature cycles, one HMB-reflector was glued on the surface of an old asphalt pavement specimen (the same material as in the Series A) and subjected to 20 temperature cycles with testing temperatures periodically varying between -15°C and $+40^{\circ}\text{C}$.
- Pull-off tests were carried out on all HMB-reflectors of the Series A at the end of the MMLS3 traffic load simulation and the cyclic temperature test. The pull-off tests were performed up to fracture, and the maximum tensile force was measured and recorded.

Infrared (IR) thermography was used with the aim of determining the position and shape of cavities and debonding defects which occurred in the MMLS3 test (Series B) under the HMB-reflectors and HMB-cover material.

2. Materials

2.1 2-component HMB-glue and 2-component HMB-cover material

The 2-component HMB-glue (ZR 070202/3) and the 2-component HMB-cover material (ZR 271000/29) are specially designed for the application of the HMB-reflectors according to the formulas of the company Coverplast. These products consist of binder, fillers and additional materials. The binders of these products are monomeric systems with methacrylic acid methylester (MMA) as major part and, if necessary, co-monomers as admixture.

2.2 HMB- reflector

The HMB-reflector (Fig. 1) has dimensions of 450x150x9 mm and is made of the plastic ABS E200. Its cross-ribs have a length of 75 mm (horizontal projection) and a width of 14 mm. The overall internal distance between the ribs is 47 mm.

3. Experiments

3.1 Testing of traffic load performance with MMLS3

3.1.1 Principle of MMLS3

The purpose of MMLS3 is the accelerated laboratory and field testing of the mechanical behavior of pavements under one-way moving dynamic traffic loads, especially the investigation of the surface courses with regard to the permanent deformation (rutting). In this case MMLS3 was also used for testing the resistance to dynamic traffic loads during the wheel passing over the reflectors, in particular the resistance to the corresponding horizontal shear loads. These dynamic traffic loads are generally assumed to produce stress-strain situations within the linear viscoelastic range [1].

The functioning of MMLS3 has been reported in detail elsewhere ([2], [3]). Fig. 2 shows the principle of the MMLS3. Table 1 reports the parameters which were chosen in these reflector tests.

3.1.2 Preparation of the test

For the MMLS3 testing two compacted asphalt pavement slabs were used:

- An old three layer slab with an asphalt concrete surface course AB11 from EMPA stock (Series A). This slab was taken more than 20 years ago from a highway with a fairly heavy traffic after many years of use. Its dimensions were 1900x750x280 mm.
- A new two layer slab with a surface course AB11 (Series B) which was paved about 2 month before testing and taken the next day. It had dimensions of 1500x800x160 mm.

Both slabs were embedded and fixed with cement mortar in open air into a pre-fabricated reinforced concrete frame. In order to allow curing of the cement mortar, the specimens were left in the open air for 14 days. The mean air temperature during daytime was about 25°C.

After curing of the cement mortar, the HMB-reflectors were applied in two steps. In a first step the reflector was glued on the pavement surface. In a second step the 2-component HMB-cover material was applied to the border zones of the HMB-reflectors and to the adjacent zones of the pavement surface. The application of the 2-component HMB-cover material was performed in 4 steps. Overall, 6 reflectors were applied, 2 reflectors for each of the tests A30 (constant testing temperature $T=30^{\circ}\text{C}$), A40 and B40 (testing temperature $T=40^{\circ}\text{C}$). One of the reflectors was arranged perpendicular to the longitudinal axis of MMLS3 (labeled with N in Fig. 3), another one was located in an angle of 45° to this axis (labeled with S in Fig. 3). The reflecting elements of the perpendicular reflectors showed towards the driving direction (corresponding to the “normal” traffic situation) and their fixation in the socket of the reflector was therefore tested in a compression mode. The reflecting elements of the 45° oriented reflectors showed away from the driving direction (Fig. 3) which had the effect that the fixation of the reflector elements in the socket of the reflector was tested in a tension mode. The MMLS3 was placed on the testing specimen as shown in Fig. 4. To achieve constant testing temperature a closed aluminum container was built around the whole testing configuration. Hence, the testing temperature during testing varied by max. $\pm 1^{\circ}\text{C}$.

3.1.3 Measurement

A special profilometer (produced by MLS Test Systems cc) was used in order to measure the rut depths. It was installed under the MMLS3 perpendicular to the driving direction (Fig. 5). For reproducibility reasons the profilometer was mounted on “reference points“. The measurement and recording of testing data was done automatically.

After placing the MMLS3 on the specimen, a zero measurement was performed for the unloaded condition as reference for the following measurements. The development of rutting in the tests A30, A40, and B40 was measured and recorded with the profilometer periodically up to 2 millions wheel passings. It was measured in transverse profiles which were located 50 mm apart from each other. The absolute measuring accuracy was ± 0.1 mm. As shown in Fig. 6 (Series A), the transverse measurements for all tests started at the origin of the chosen coordinate system. The distance between the transverse measurement points was 5 mm. After determination of each profile, the next profile in the longitudinal direction towards the driving direction was measured. In this way, 12, 11 and 11 profiles were recorded for the tests A30, A40 and B40, respectively.

3.2 Resistance to temperature cycles

In order to investigate the resistance to temperature cycles, a specimen from Series A was prepared. Again, a HMB-reflector was glued on its surface with the 2-component HMB-glue and covered with the 2-component HMB-cover material. The horizontal dimensions of the specimen were 600x300 mm. The prepared specimen was thermally insulated at the sides with polystyrene panels. After that, the specimen was laid on a wooden plate and placed into a temperature chamber. The temperature was measured with a temperature sensor which was attached to the surface of the applied reflector.

In this test 20 trapezoidal temperature cycles were performed. The testing temperature varied linearly during 90 minutes between -15°C and $+40^{\circ}\text{C}$ followed by a constant temperature phase of another 90 minutes. Hence, one full testing cycle took 6 hours.

3.3 Pull-off tests

For the preparation of the pull-off tests the chosen testing location had to be leveled out mechanically. For this reason the middle ribs of the HMB-reflectors were removed with a heated putty-knife. After that the circular cuts were made with a 50 mm core. Depending on the testing location the circular cut reached the pavement surface. A 50 mm circular steel plate was glued on the testing location with a 2-component glue. After curing of the glue the steel plate was connected to the servo hydraulic testing machine and vertically pulled with a load controlled velocity of 200 N/s (Fig. 7). All pull-off tests were carried out at room temperature of about $T=23^{\circ}\text{C}$ up to fracture, and the maximum tensile force was measured and recorded.

3.4 IR-Thermography

3.4.1 Inspection principle

At EMPA, passive thermography is used as a thermal non-destructive testing method in search for hidden defects and/or damages in a road or bridge pavement structure (layered structure), based on the use of a heat flow to investigate their inner structure. The conduction heat flow q in the z direction along the object thickness is described by Fourier's law:

$$q_{\text{Conduction}} = -k * \partial T / \partial z .$$

The method exploits the insulation effect by thermal diffusion caused by the defects and discontinuities, i.e. the thermal resistance ($R_w = L * k^{-1} * A^{-1}$; [K W⁻¹]) of the defect is higher than the areas with no defects in the sample. Hence, the air trapped in the delaminated defects (typical for example for adhesion problems in a layered structure) acts thermophysically as an insulation region; disturbing the heat flow in the specimen ($k_{\text{Air}}; T=20^\circ\text{C}; p=1\text{bar} = 26.03 \times 10^{-3} \text{ Wm}^{-1} \text{ K}^{-1}$; $k_{\text{Pavement}} = 0.53 \dots 1.87 \text{ Wm}^{-1} \text{ K}^{-1}$). Therefore, a temperature difference ($\Delta T = T_d - T_{nd}$) develops at the surface between the hotter region above the defect T_d and its non-defect surroundings T_{nd} that can be detected with the IR-camera. Depending on the observation technique (transmission, reflection), the temperature difference can be positive for the transmission technique, or negative for the reflection technique.

In fact, two methods of observation are possible: in reflection, both the thermal source and the detector unit are located on the same side of the work piece; in transmission, the thermal source and detector unit are located on opposite sides of work piece [4].

IR thermography can also be used to display the location and the planar shape and extension of the defects buried in the specimen. Usually, a sample is in a state of thermal balance with the ambient. Therefore the thermal balance must be altered through a heating or cooling method in order to induce a thermal gradient and to allow an IR investigation. The key to a successful inspection of the delaminated defect is a correct choice of the type, intensity and duration of this thermal alteration as well as of the timing of the recorded signal.

When the sample is heated or cooled rapidly, a strong warm or cool flow can lead to a very abrupt change of the temperature field on the sample surface and to a higher inspection sensitivity. The heating or cooling method must generate a uniform surface temperature field $T(x,y,t)$ on the surface of a sample in order to obtain a uniform heat flow in the sample.

3.4.2 Preparation and IR measurement

During the recording of the thermograms the cooling down technique has been used. This technique consists of taking the thermograms during the cooling phase of the investigated area. In this case the cooling occurs through the natural convection. In a sample with a layered structure it is favorable that the layers have a constant thickness ($L = \text{const}$) otherwise the surface temperature field does not appear uniform in the thermogram (this is also quite evident from R_w).

In order to apply the cooling down IR measurement method, in this study, the specimen was first heated up with a heat transfer medium placed above the upper surface of the specimen. The heat transfer medium consisted of a plastic sheet and one layer of crushed sand 0... 3 mm

with a thickness of approximately 80 mm. The crushed sand was heated during a week-end in a furnace at approximately 80 °C. This medium was able to store and transfer the heat to the upper surface of the specimen within approximately 20 minutes. The medium was also able to adapt to the surface unevenness of the specimen in order to guarantee a good thermal contact between the two surfaces. Moreover it was easy and quick to remove.

In order to slow down the cooling due to convection, the remaining exposed parts of the heat transfer medium were cautiously covered with polystyrene panels.

The infrared measurement system for this investigation consisted of an infrared camera (IR-camera) AGEMA THV 900 LW (Long Wave, of 8 to 12 μm) with lens 40° x 20°.

A system of thermocouple and slab of copper was placed in the middle of the upper surface of the sample (see TC in Fig. 8) for monitoring the temperature between the heat transfer medium and the upper surface of the specimen.

4. Results and Discussion

4.1 MMLS3 test results

Figs. 9 and 10 show the development of the maximum rut depths in Series A with growing number of wheel passings. The rut depths were recorded in the longitudinal profiles in the middle of the rut at a distance of 50mm. Table 2 contains the values of the maximum rut depths for all performed MMLS3-tests.

It can be clearly seen that for both tests of Series A after approx. 0.5 millions wheel passings there is practically no increase in rut depth. In addition, the development of rutting at 40°C is considerably faster than at 30°C, producing results which are apparently less influenced by local disturbances like in case of the abnormal increase of deformation under the diagonally applied reflector A30S. This was one of the reasons why Series B was only tested at 40°C. For Series A the visual observation of the tested HMB-reflectors and the 2-component HMB-cover material in the ruts and in the border areas showed no local damages. Furthermore no loss of adhesion between the HMB-glue layer and pavements surface was observed.

Fig. 11 shows the development of the maximum rut depths for Series B at 40°C. Rut depths measured in this series after 50'000 wheel passings show similar values as in test A40 (Fig. 10). However, the rut depth after approx. 500'000 wheel passings is considerably smaller. After 1'000'500 wheel passings the measured values of the rut depths are even smaller then after 50'000 passings at the beginning of the test. The reason for this is the elastic recovery of the surface after debonding of the HMB-reflectors and HMB-cover material from the pavement surface (Fig. 12).

4.2 Pull-off test results

Results of the pull-off tests at room temperature are summarized in Table 3. In this table the tests T1 to T3 were performed on the reflector on the specimen of the resistance to temperature cycles test; the tests A30Sx, A30Nx, A40Sx, A40Nx correspond to the reflectors of the MMLS3-Series A. It is obvious that the failure stresses for the old pavement specimen is practically independent of the application direction of the reflector and the MMLS3 testing temperature. On the other hand, a clear influence of temperature cycles on the pull-off strength appears to exist. As compared to the MMLS3 specimens, the reduction of the pull-off failure stress is more then 20%.

The failure zone in all pull-off tests was located in the surface course and not in the HMB-glue layer.

4.3 IR test results

The aim of this thermal non-destructive evaluation was the detection of possible defects and discontinuities through a qualitative comparison of the surface temperature signals caused by them. This required a comparison of the local signals of the possible defects with those of areas with no defect.

Fig. 8 shows the thermogram and the visual image of the specimen. The investigated surface has dimensions of 800 x 1500 mm. The thermogram was taken shortly after removal of the heat transfer medium from the heated specimen surface. For better visualization, the values of the surface temperature were visualized and analyzed with a Matlab application.

From the thermal image it is found that the surface temperature of the zones with no defect ranges between 27.0 °C and 27.7 °C. The HMB-reflectors are easily recognized by their shape (see Fig. 1 and Fig. 8). In addition, six regions of interest (ROI) B1 to B6 appear which were supposed to be delamination zones. This was verified by a longitudinal cut between approximately 250 and 280 mm from the upper edge of the investigated specimen. The longitudinal cut crosses the ROIs B1 to B4 and cores of the ROIs B5 and B6 were also taken.

Table 4 reports the parameters of the average surface temperatures (\bar{T}) and the geometrical factors measured along the section.

It is concluded that B1 to B4 are delaminations (see Fig. 12), while B5 and B6 are caused by the thickness discontinuity of the cover and of the HMB-reflector shape.

Overall it can be concluded that information about location, planar shape and extension of the defect area could be successfully obtained directly from the infrared image. However, false signals (B5, B6) appeared due to the non-uniform thickness.

5. Conclusions

This study led to following main conclusions and findings:

- The model mobile load simulator (MMLS3) can be applied successfully for the optimization and evaluation of reflector systems as described in this study with respect to selected performance related mechanical properties, such as adhesion of the reflector system to the asphalt pavement surface, rutting behavior and mechanical resistance of reflectors to traffic loads.
- For the reflector system investigated herein, it was found, that adhesion is significantly dependent on the surface properties of the pavement. Whereas the reflector system on an old asphalt pavement with a comparatively high amount of uncoated aggregate on its surfaces showed very promising results, the reflector system on a new asphalt pavement with a comparatively high amount of coated aggregate on its surface suffered debonding failures.
- Rutting in the MMLS3 tests at 40°C was considerably faster than at 30°C and was apparently less influenced by local disturbances.
- The orientation of the reflectors with respect to the driving direction had no significant influence on rutting.
- Pull-off failure stresses at room temperature after rutting tests were practically independent of the application direction of the reflector and the MMLS3 testing temperature.
- As far as the resistance to temperature cycles is concerned, a clear influence on the pull-off strength appears to exist. As compared to the MMLS3 specimens, the reduction of the pull-off failure stress was more than 20%.
- Using the cooling down method, IR-thermography proved successful in providing useful information about location, planar shape and extension of the debonding defects on a new pavement during MMLS3 rutting.

References

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Figure 1 HMB-reflector: generally view (a); applied on the road surface (b).

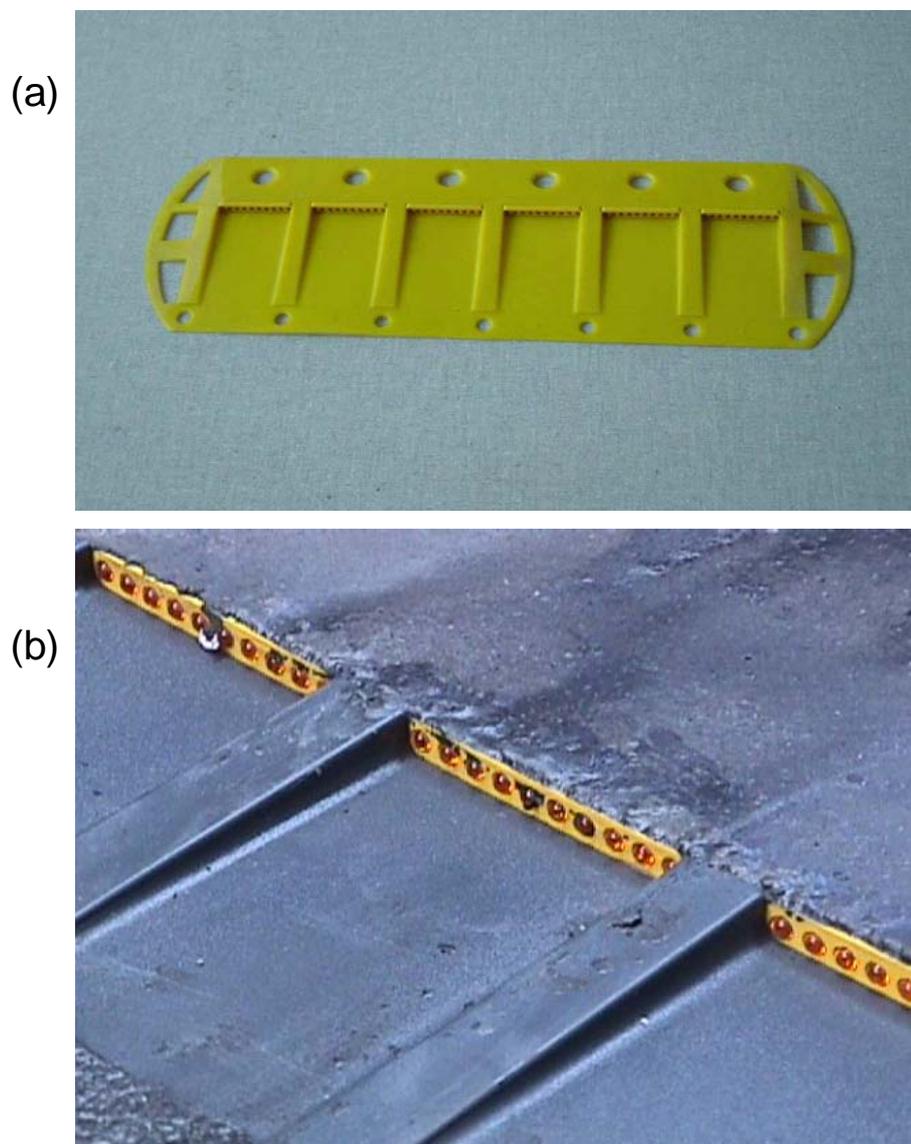


Figure 2 Diagram of MMLS3.

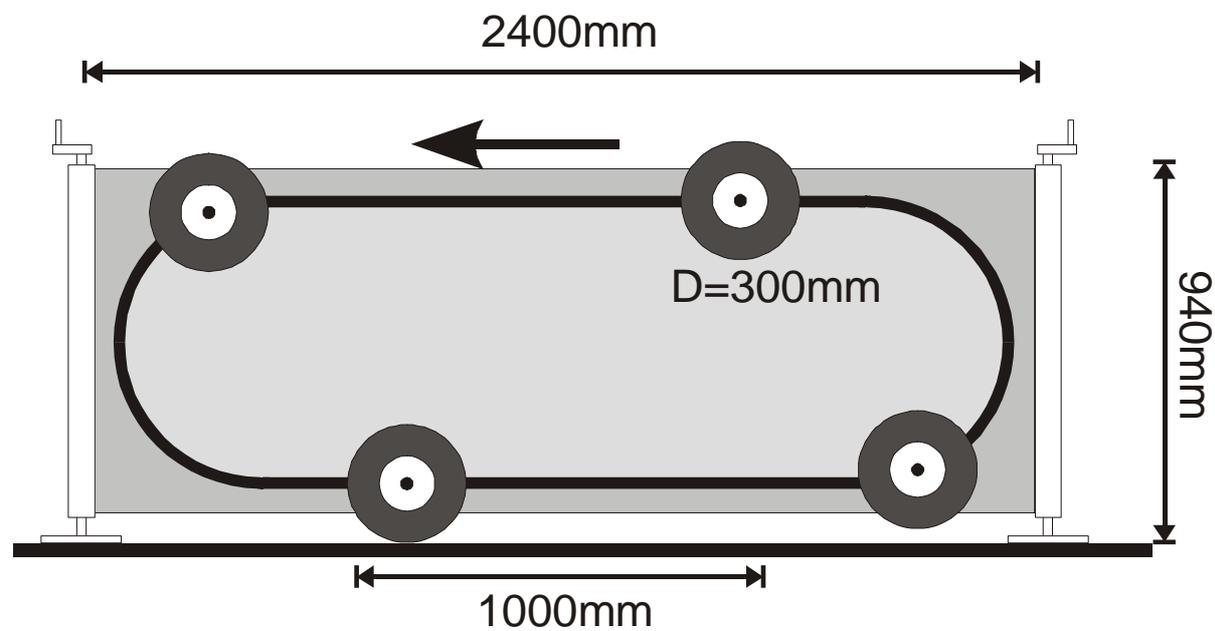


Figure 3 Arrangement of the HMB-reflectors (series A): general view of the testing slab and reflectors labels (a); reflector A30N (b); reflector A30S (c).

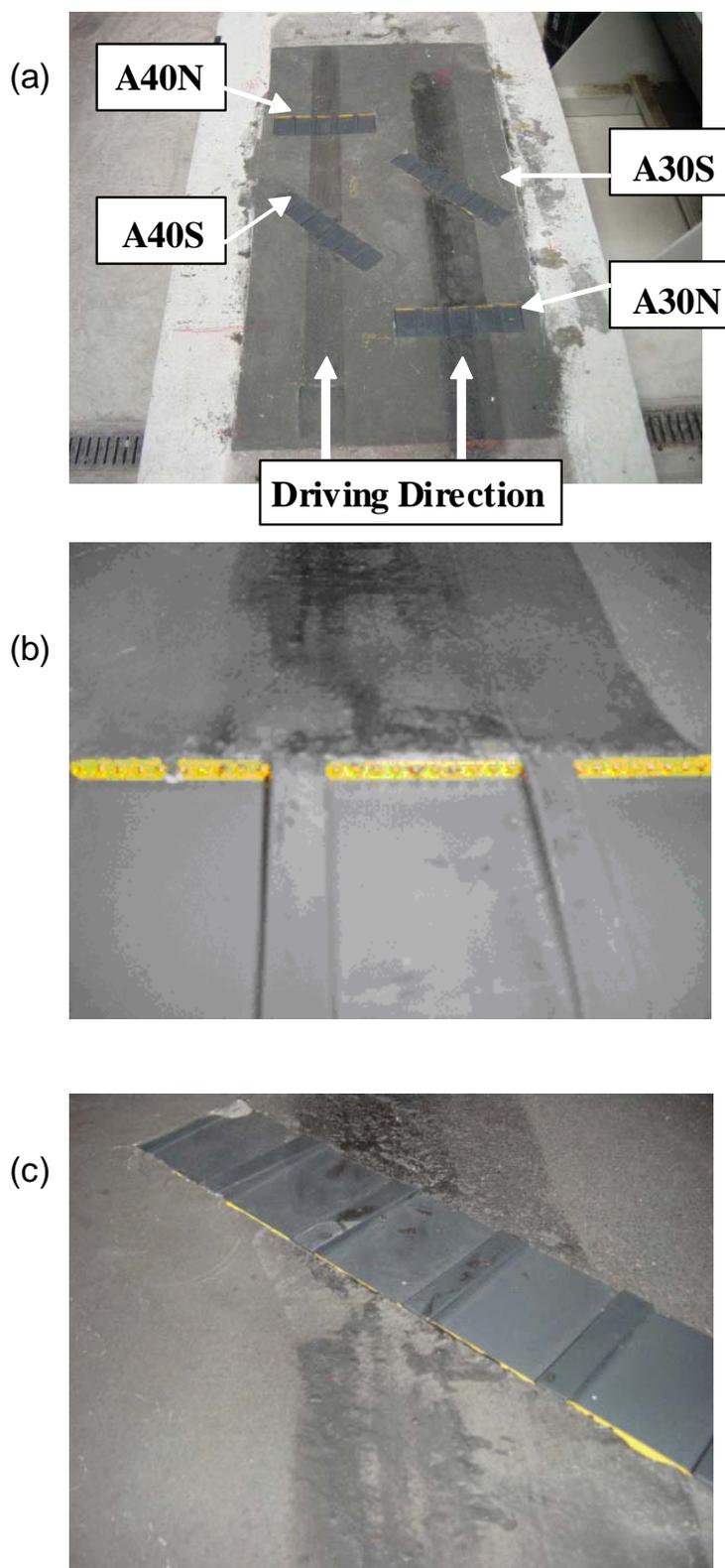


Figure 4 MMLS3, placed on the specimen (series A).

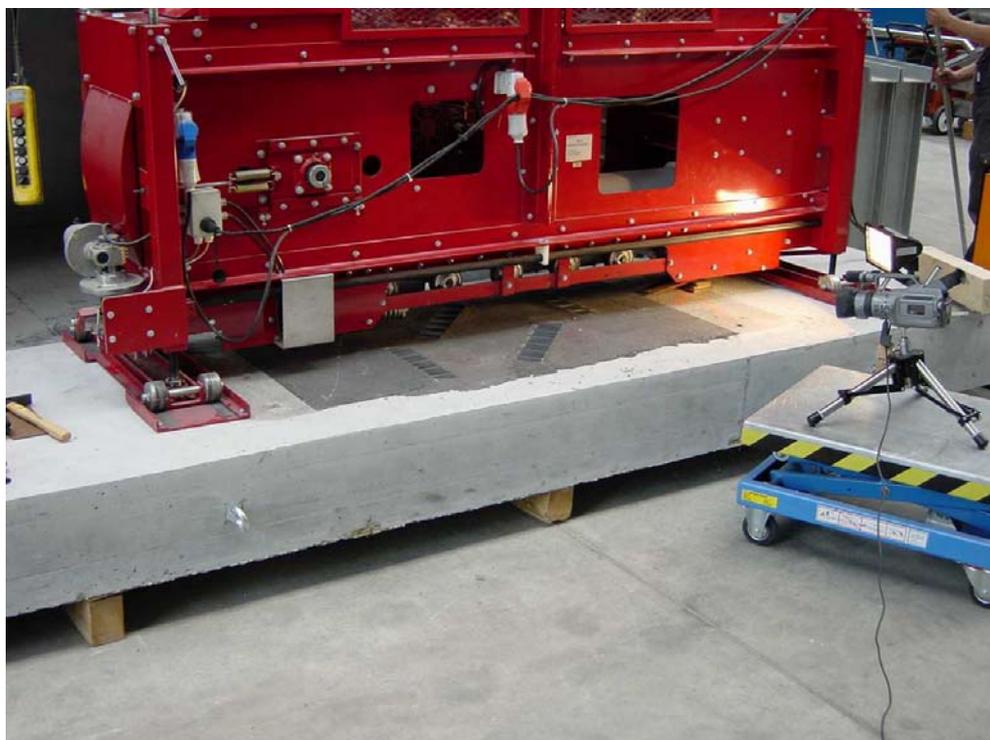
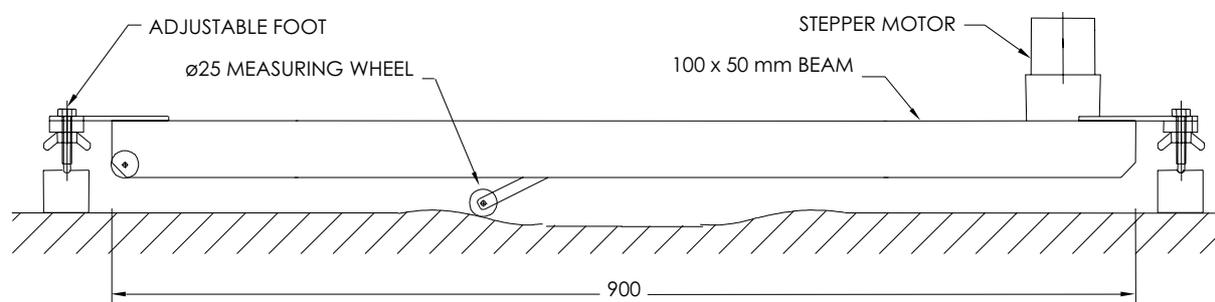


Figure 5 MMLS3 profilometer.



Source: MLS Test Systems cc

Figure 6 Measuring area (test A30).

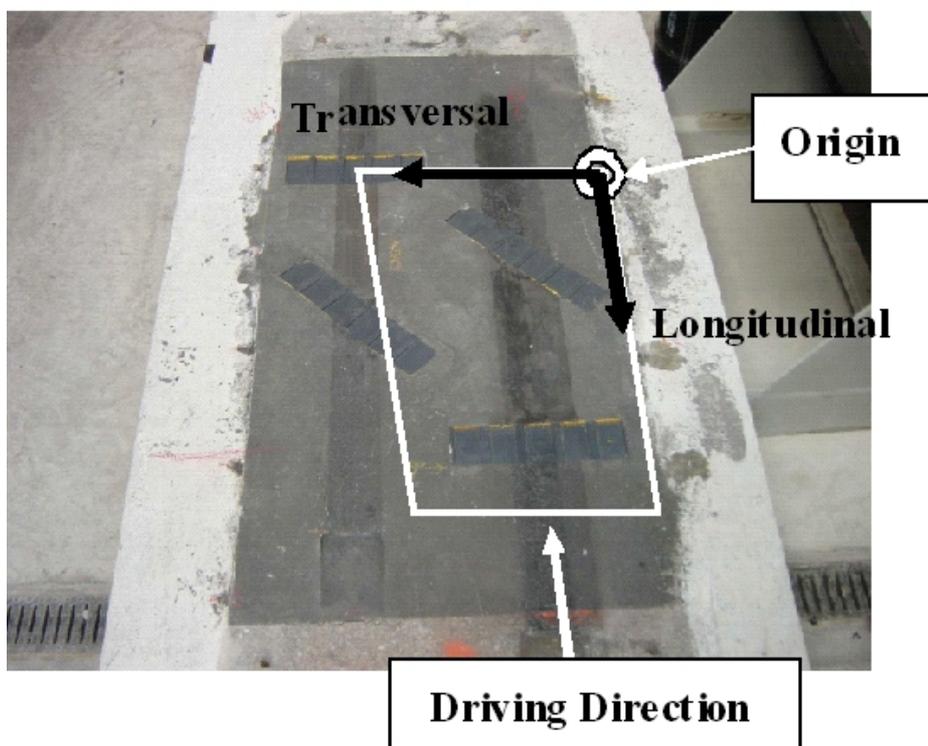


Figure 7 Pull-off test equipment, working zone.



Source: MLS Test Systems cc

Figure 8 Visual and thermal images of the specimen (cover and HMB-reflectors).

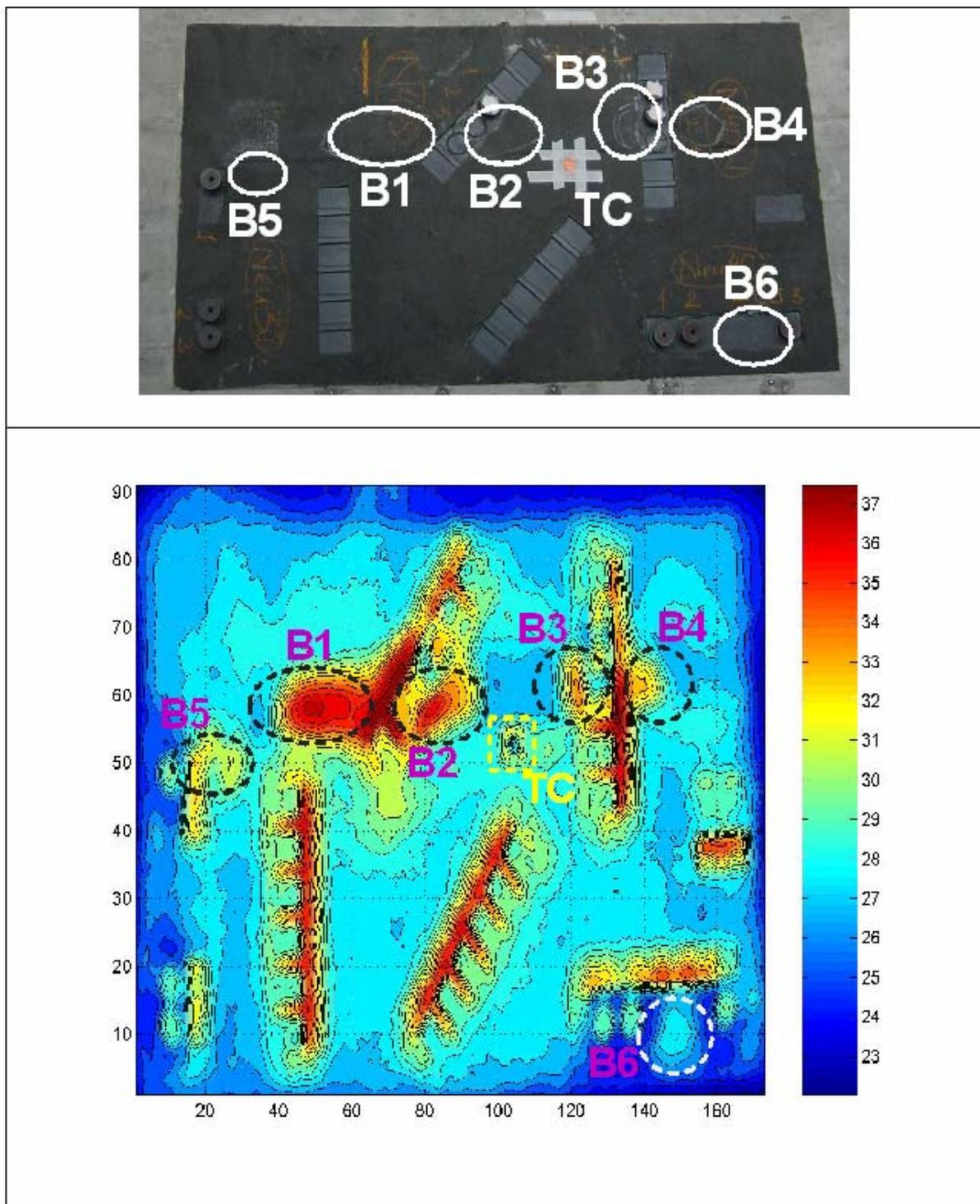


Figure 9 Rut depths, experiment A30: longitudinal profile in the middle of the rut.

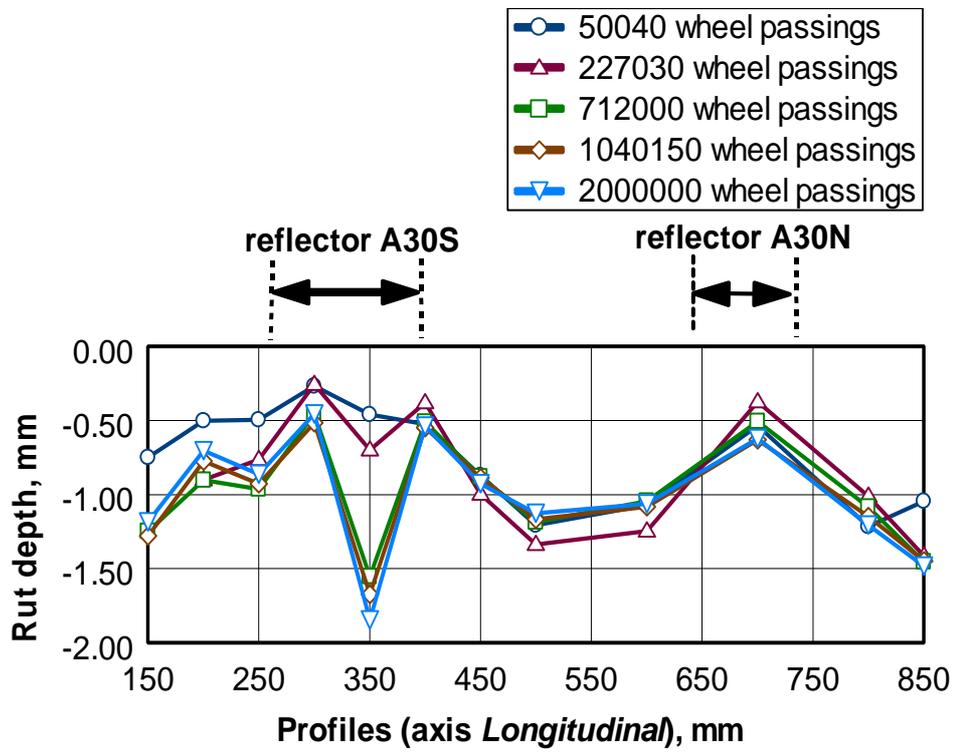


Figure 10 Rut depths, experiment A40: longitudinal profile in the middle of the rut.

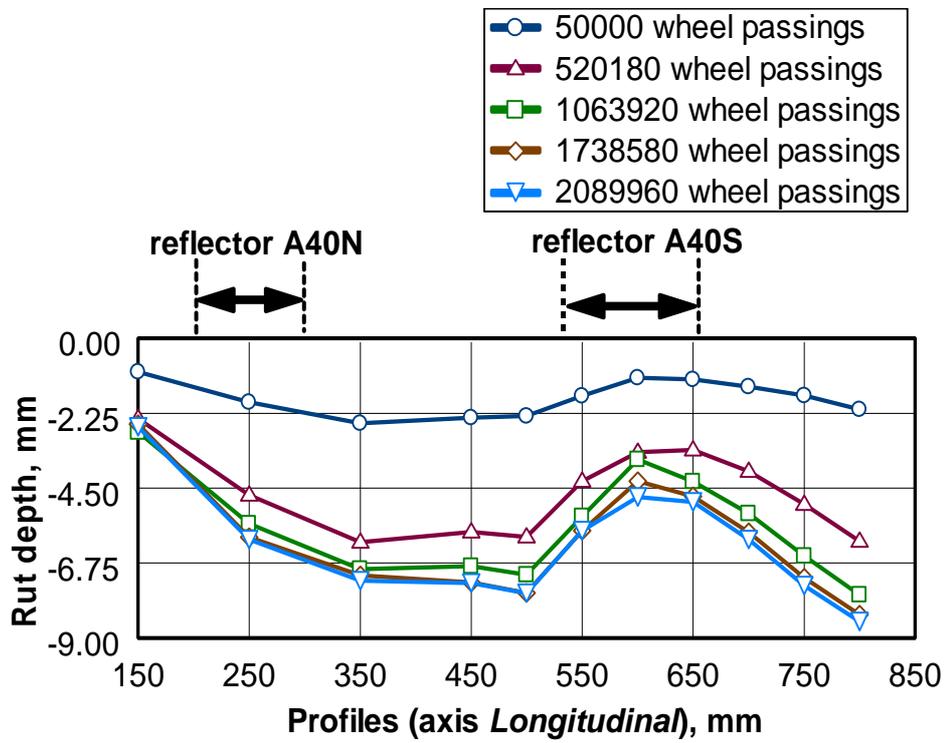


Figure 11 Rut depths, experiment B40: longitudinal profile in the middle of the rut.

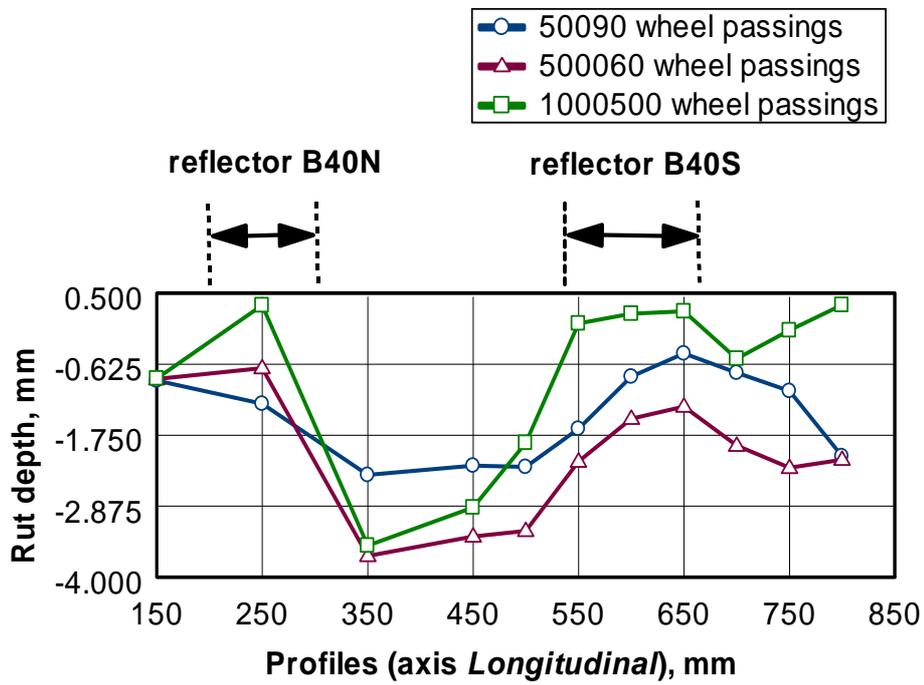


Figure 12 Vertical section of the longitudinal cut (B1U, B2U, B3U, B4U).

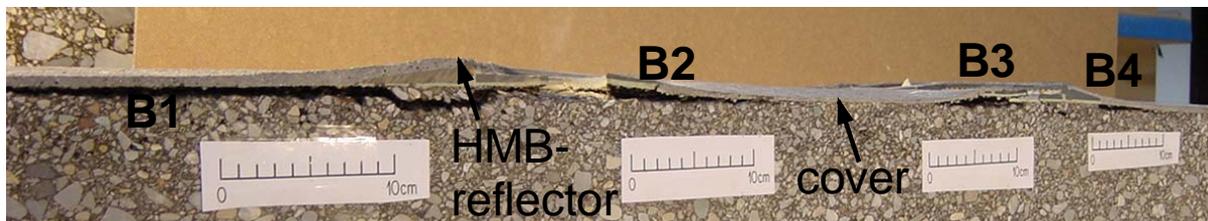


Table 1 MMLS3: technical specifications relevant for the reflector tests.

Wheel diameter (mm)	Tire width (mm)	Tire footprint area (cm ²)	Tire contact pressure (kPa)	Nominal load per wheel (N)
30	80	34	620	2100

Table 2 MMLS3 tests, measured rut depths.

Test	Wheel passings	Maximum rut depth (mm)
A30 (t = 30°C)	50040	-1.347
	227030	-2.025
	712000	-2.779
	1040150	-3.252
	2000000	-3.765
A40 (t = 40°C)	50000	-2.678
	520180	-6.249
	1063920	-8.039
	1738580	-8.740
	2089960	-8.996
B40 (t = 40°C)	50090	-2.377
	500060	-3.660
	1000500	-3.496

Table 3 Results of the pull-off tests (room temperature).

Test	Steel plate diameter (mm)	Maximum load (kN)	Pull-off failure stress (MPa)	Mean value of the pull-off failure stress (MPa)
T1	49.2	2.98	1.567	
T2	48.3	3	1.637	1.620
T3	49.3	3.16	1.655	
A30S1	49.2	4.38	2.304	
A30S2	49.3	3.78	1.980	2.071
A30S3	49.4	3.7	1.930	
A30N1	49.5	4.04	2.099	
A30N2	49.5	3.9	2.027	2.061
A30N3	49.4	3.94	2.056	
A40N1	49.2	4.14	2.178	
A40N2	49.2	3.42	1.799	2.020
A40N3	49.3	3.98	2.085	
A40S1	49.3	3.42	1.792	
A40S2	49.2	3.92	2.062	1.924
A40S3	49.3	3.66	1.917	

Table 4 Summary of six regions of interest (ROIs).

Longitudinal cut					
Denomination	Length (cm)	Height (cm)	Depth (cm)	\bar{T} (°C)	Comment
B1U	41.0	0.60	0.50	35.9	
B1D	35.0	0.60	0.50		Depth below HMB-reflector = 2.0 cm
B2U	19.0	0.70	0.90	34.4	Depth not constant
B2D	13.0	0.80	0.50		Depth below HMB-reflector = 0.80 cm
B3U	11.8	0.25	0.50	32.5	
B3O	9.0	0.60	0.50		Depth below HMB-reflector = 0.70 cm
B4O	9.6	0.30	0.50	32.0	
B4U	8.3	0.30	0.50		
Core					
Denomination	\bar{T} (°C)	Comment			
B5	30.7	Cover thickness 4 to 10 mm			
B6	27.8	Cover thickness 4 to 7 mm			